



Capacity drop in Belgium: empirical observations and implications

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TRAIL masterclass Mysteries on motorway traffic operations
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Overview

- Part 1: capacity drop on Belgian motorways
 - R0 ring of Brussels
 - empirical observations
 - implications for potential impact control applications
- Part 2: capacity drop on Belgian intersections
 - concept and definition
 - empirical set-up
 - empirical observations

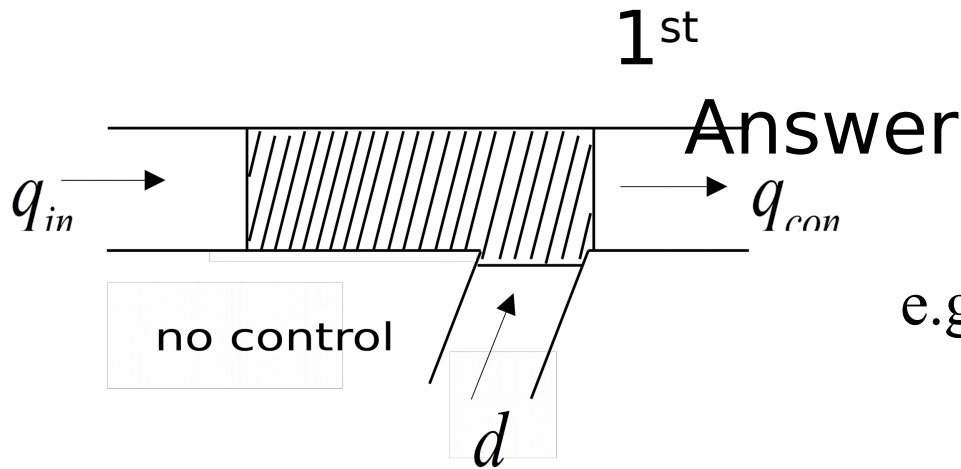
Capacity drop on Belgian motorways

master thesis Thijs Van Tieghem
KU Leuven master Traffic & Logistics

Motivation and objective

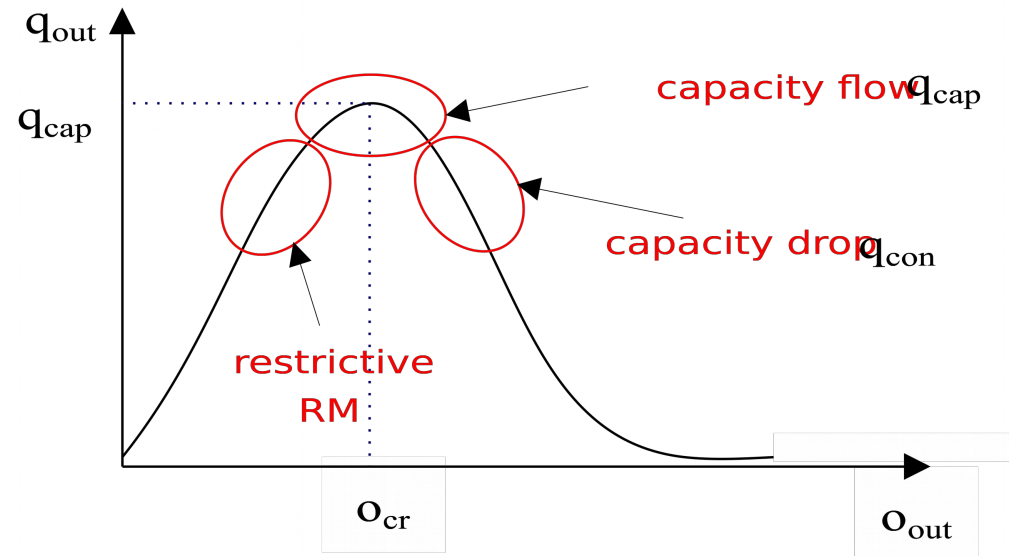
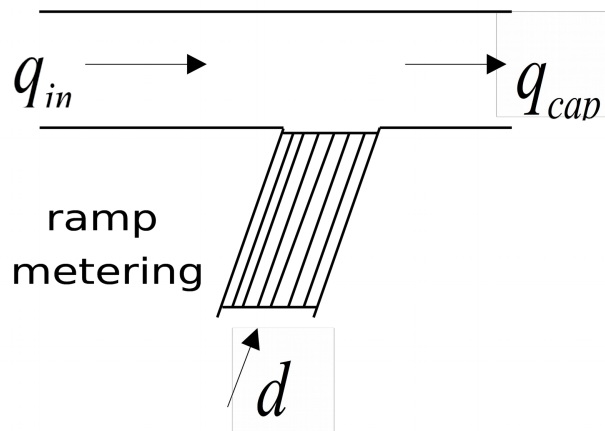
- potential for traffic control claimed
 - literature
 - capdrop = one of two main degradation mechanisms
 - e.g. work Papageorgiou predicts ~50% TTS reduction by eliminating capacity drop and spillback
 - if even 20% of that were true...
 - field deployment
 - coordinated metering in Melbourne confirmed spectacular gains
 - e.g. John Gaffney (VicRoads)
- this finally convinced Flemish road authority to support study into potential for ramp metering in Belgium

2. Why Ramp Metering?



$$\Delta T_s = \frac{T_s^{nc} - T_s^{rm}}{T_s^{nc}} = \frac{q_{cap} - q_{con}}{q_{in} + d - q_{con}}$$

e.g. $q_{con} = 0.95q_{cap}$; $q_{in} + d = 1.2q_{cap}$
 $\rightarrow \Delta T_s \text{ A } 20\%$

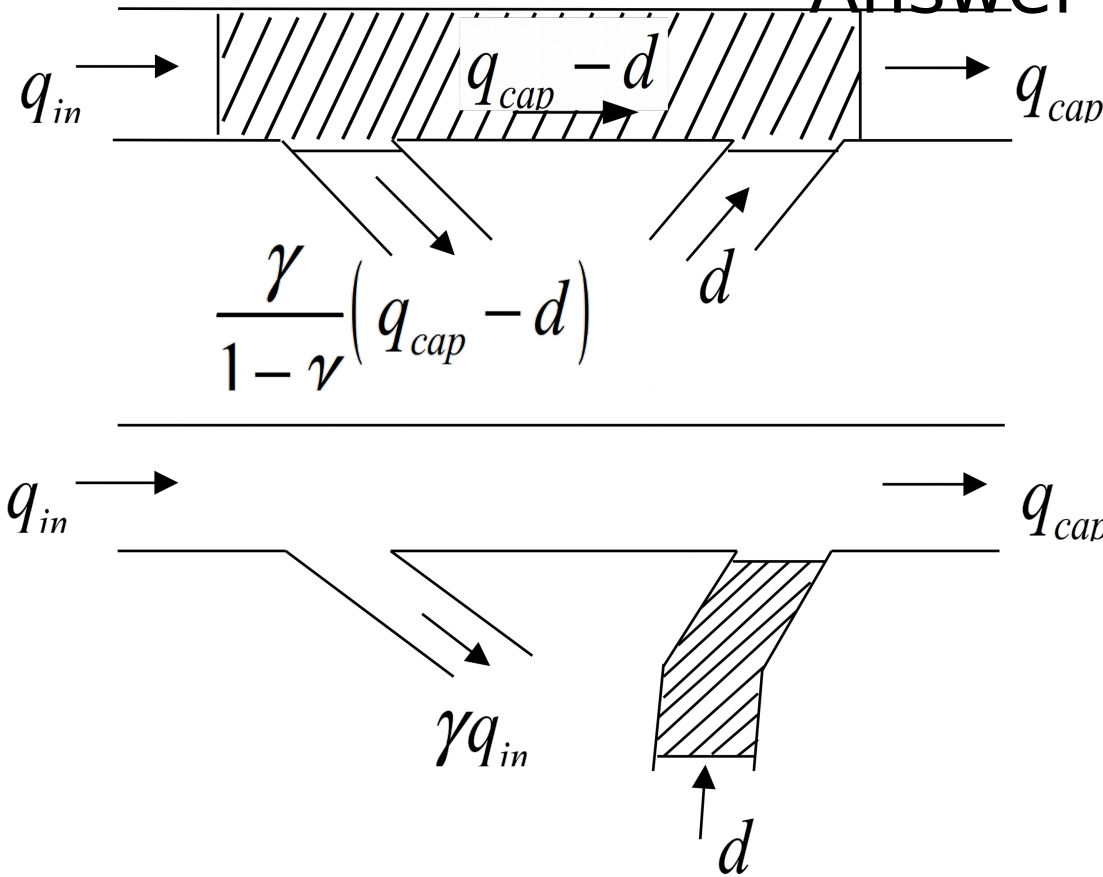


prof. Markos Papageorgiou (TU Crete) is kindly acknowledged for sharing these slides!



2nd

Answer



$$\Delta T_s = \frac{T_s^{nc} - T_s^{rm}}{T_s^{nc}} = \gamma$$

Note: On-ramp queue should not interfere with surface street traffic.

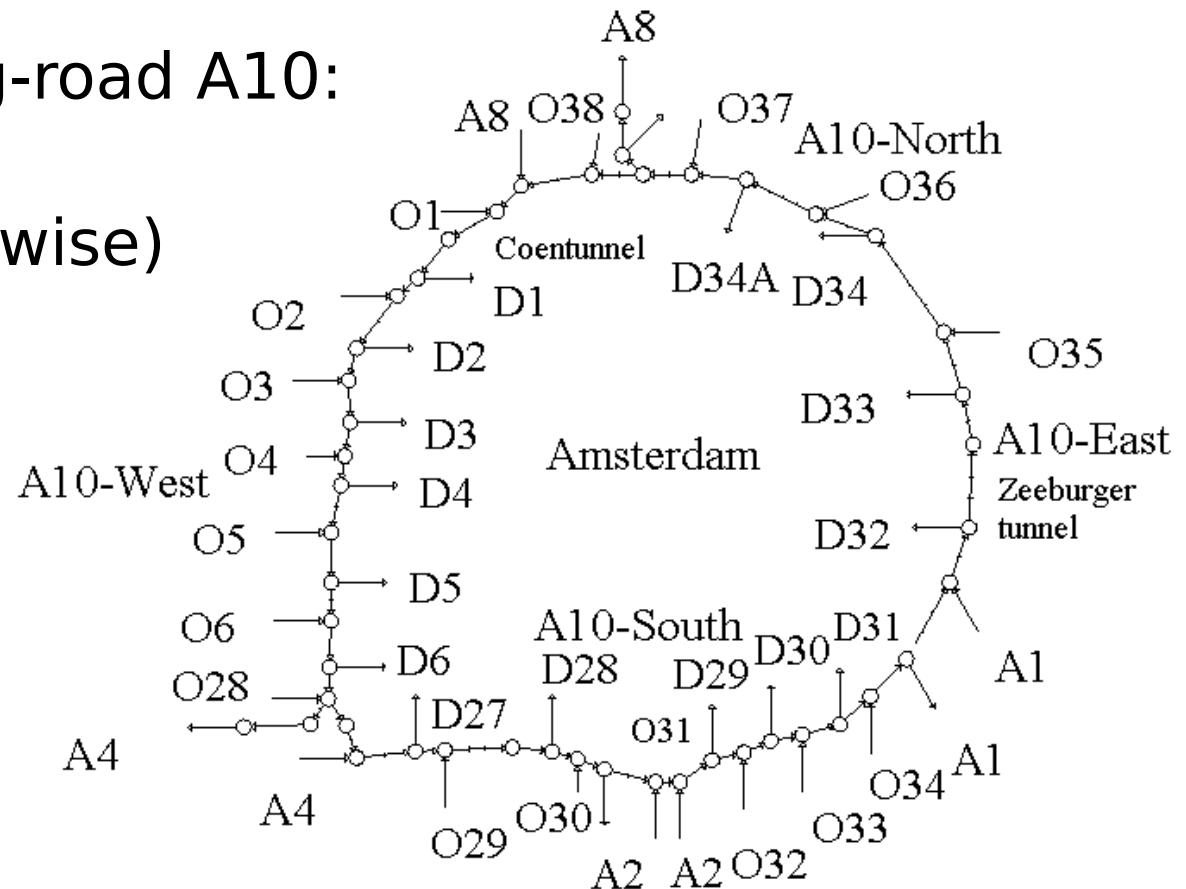
Coordinated Ramp Metering Strategy HERO: Field Implementation at the Monash Freeway in Melbourne, Australia



6. Simulation Results

Amsterdam ring-road A10:

- 32 km (counterclockwise)
- 21 on-ramps
- 20 off-ramps



Coordinated Ramp Metering Strategy HERO: Field Implementation at the Monash Freeway in Melbourne, Australia

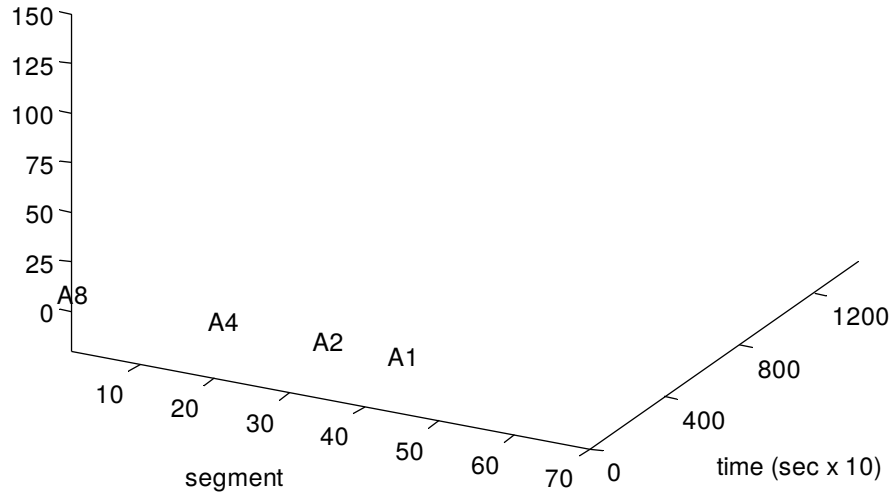


No-Control Case

TTS = 14167 veh•h

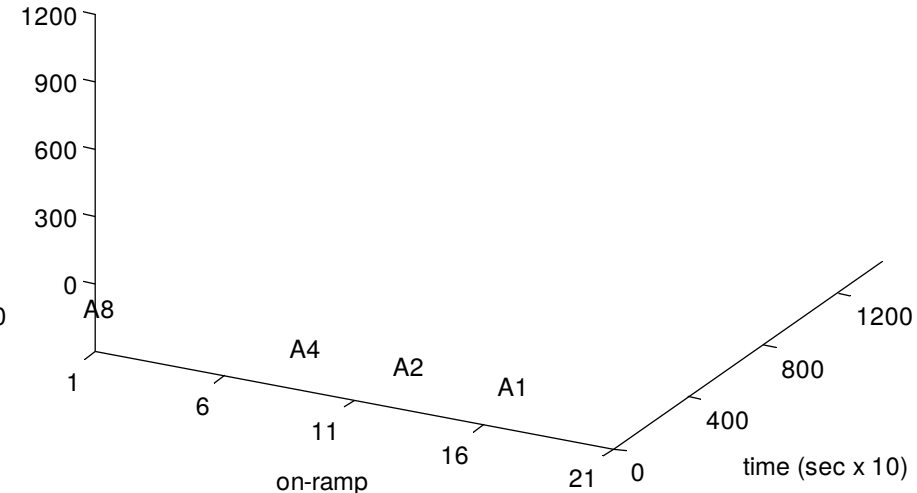
Density profile

density (veh/km/lane)



Ramp queue profile

queue (#veh)



Coordinated Ramp Metering Strategy HERO: Field Implementation at the Monash Freeway in Melbourne, Australia

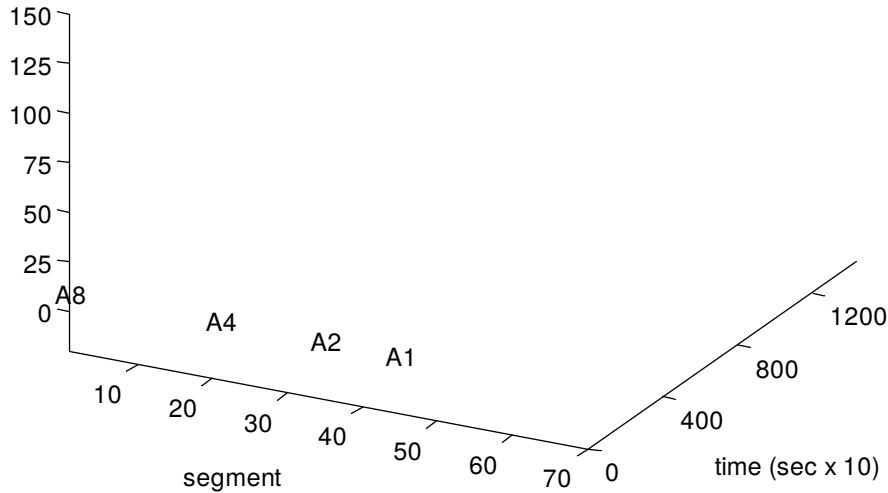


ALINEA without Queue Control

TTS = 7563 veh•h -47%

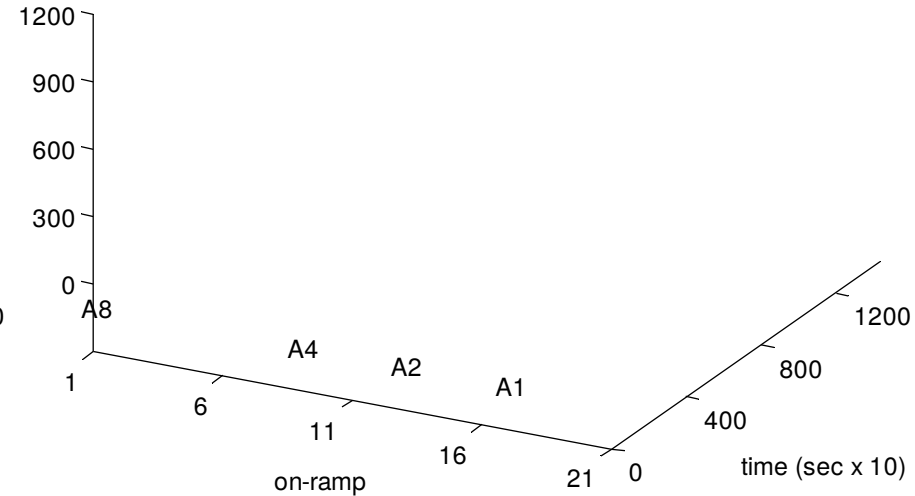
Density profile

density (veh/km/lane)



Ramp queue profile

queue (#veh)



Coordinated Ramp Metering Strategy HERO: Field Implementation at the Monash Freeway in Melbourne, Australia



Motivation and objective

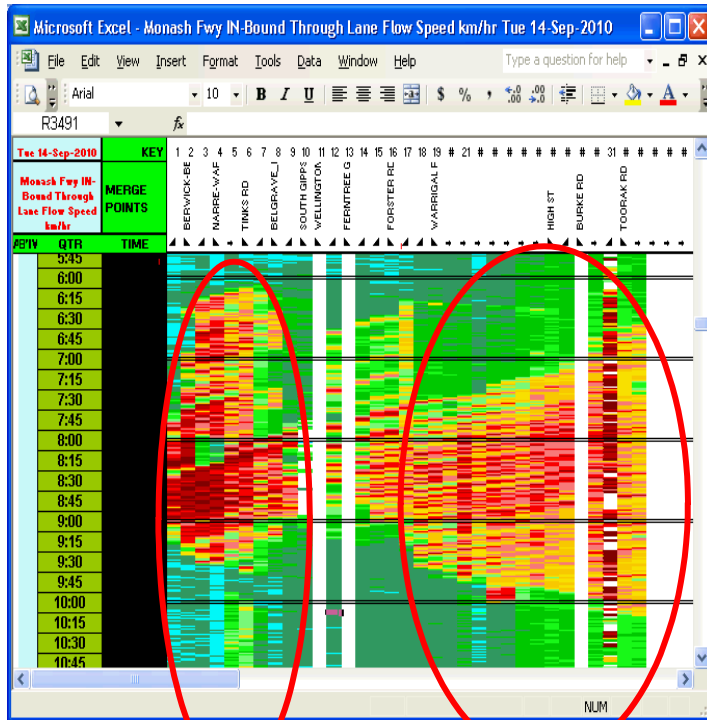
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M1 Corridor – what are the traffic patterns?



Getting Control of the Freeway

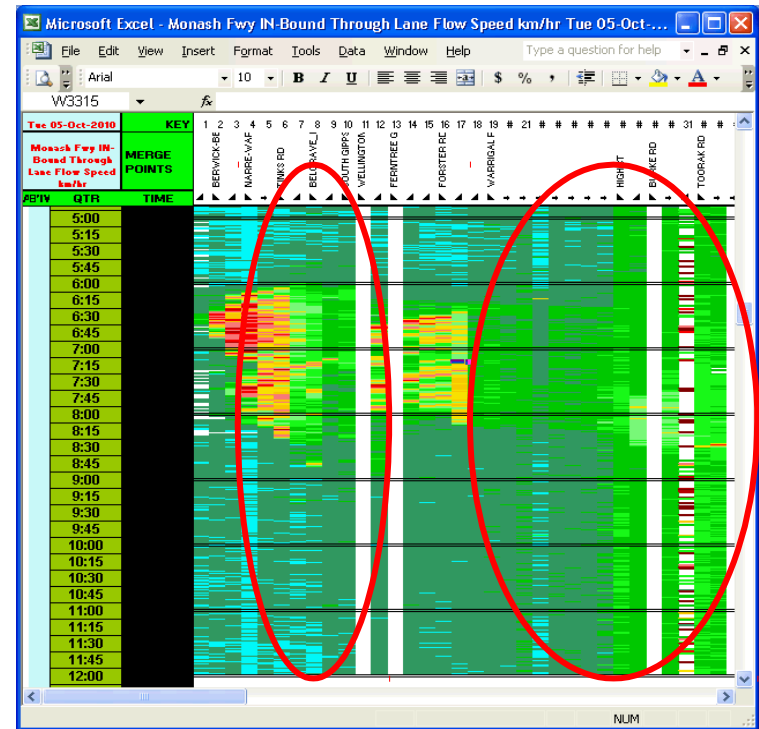
Before



2nd Bottleneck

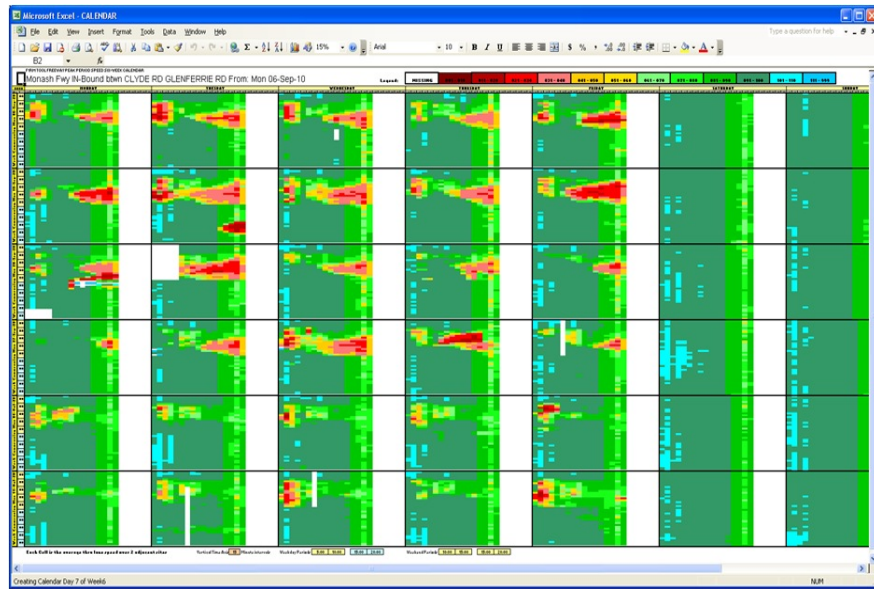
1st Bottleneck

After

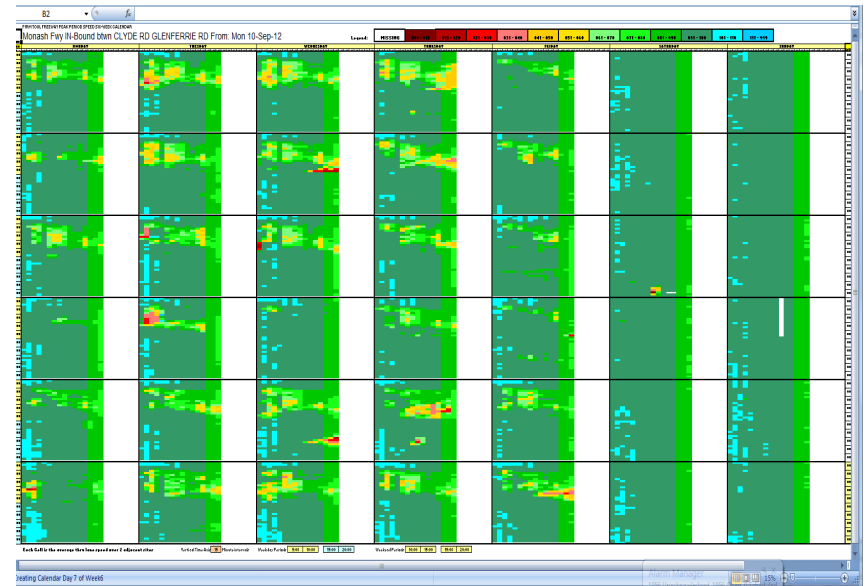


2nd Bottleneck Reduced 1st Bottleneck Removed

Before



After



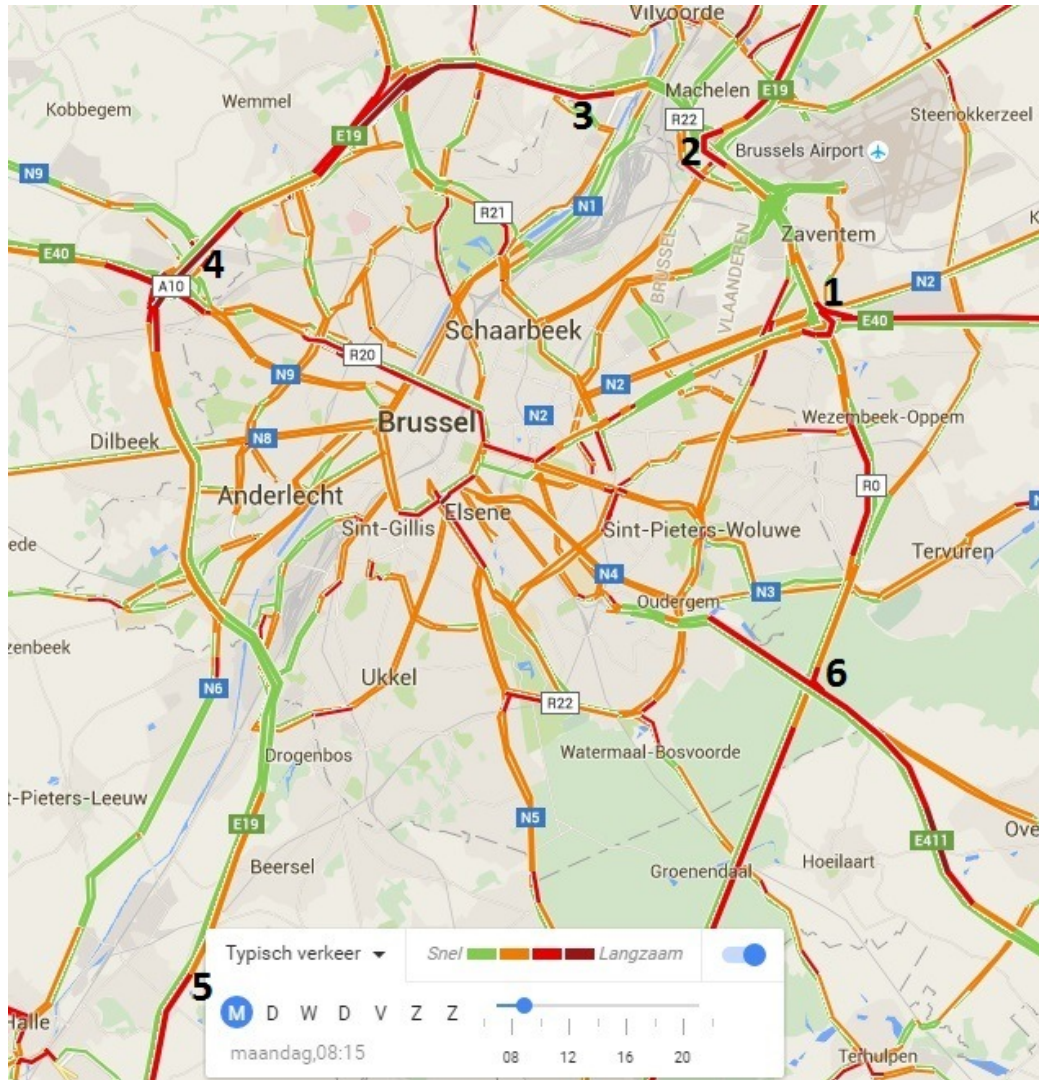
Empirical set-up

- R0 ring of Brussels
 - massive daily queuing on ring and connecting radial motorways, AM and PM
 - succession of on-off ramps with much exchange of traffic
 - many bottlenecks \Rightarrow capacity drop?
 - many blocked outflows
 - little support for capacity extension
- Research question:
“can we quantify the drop in capacity during bottleneck activation, as an indicator for potential of managed motorway?”



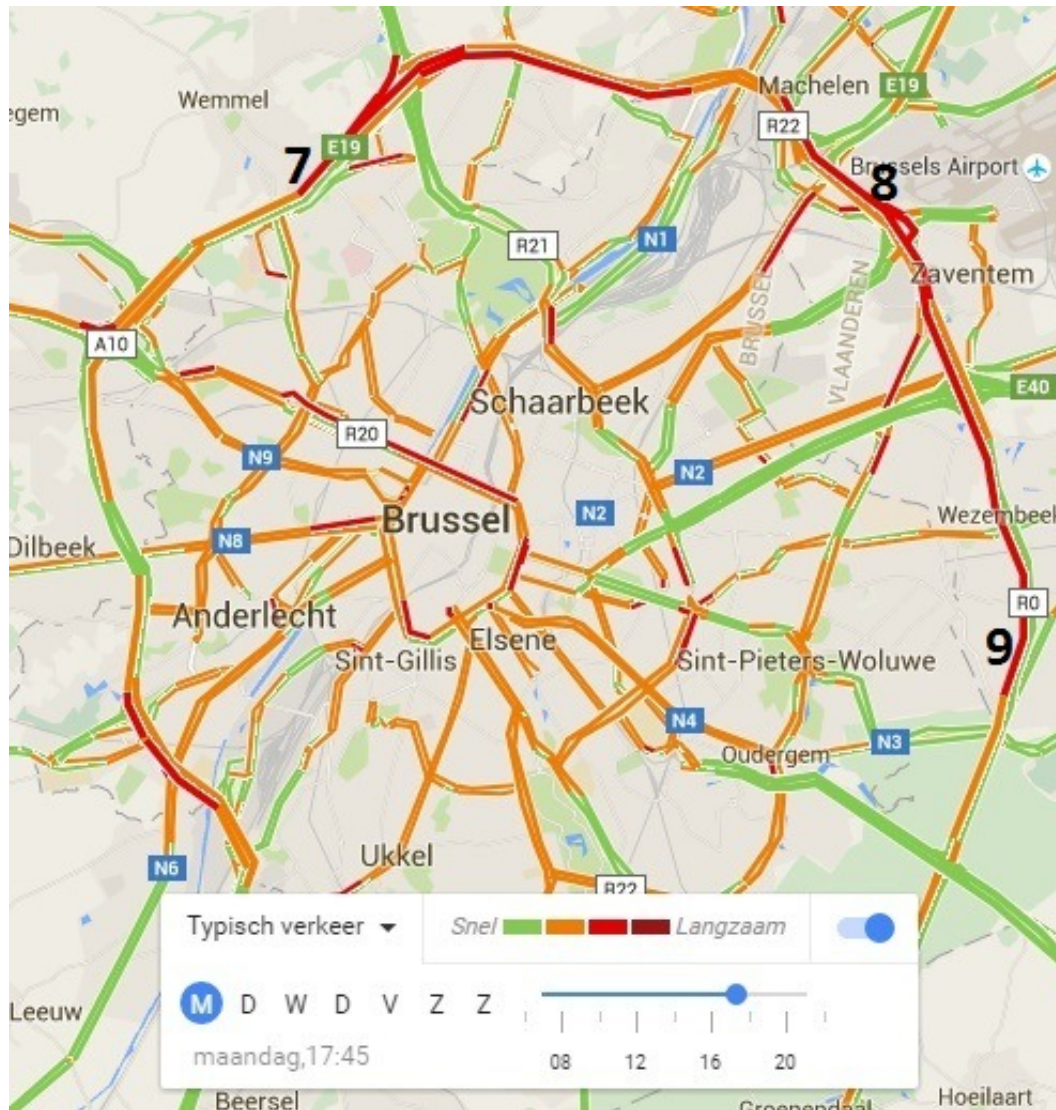
40 km

Queues AM



1. E40 ⇨ Buitenring, Sint-Stevens-Woluwe
2. E19 ⇨ Binnenring, Machelen
3. Binnenring, Vilvoorde
4. Binnenring, Zellik
5. Binnenring, Huizingen/Halle
6. E411 ⇨ Buitenring, Leonardkruispunt

Queues PM



7. Buitenring, Wemmel

8. Buitrenring, Zaventem

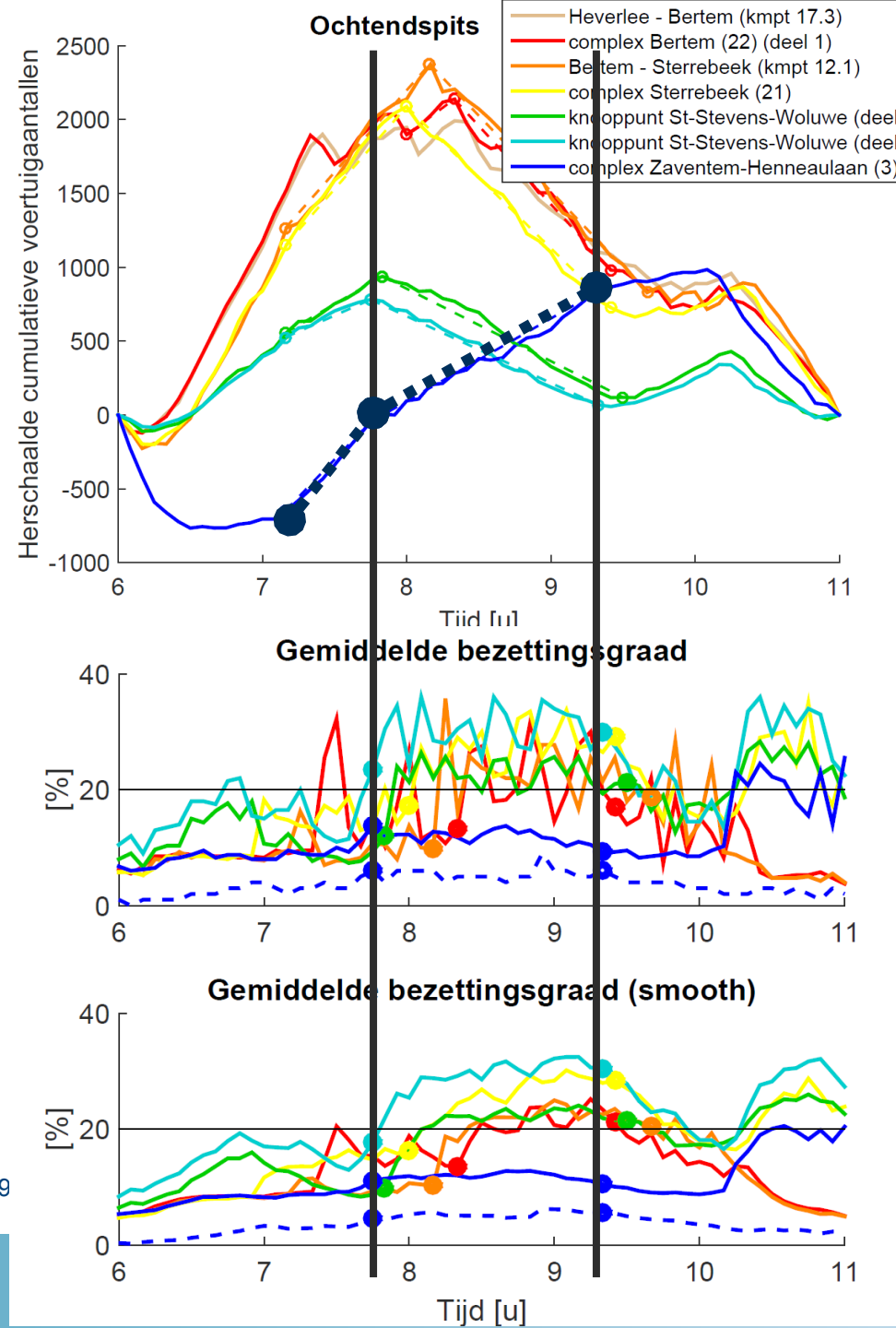
9. Binnenring, Wezembeek-Oppem

Data availability and methodology

- double loop detector data
 - 44 working days March & October 2015
 - speed, occupancy + flow per lane
 - 5-min aggregate
- methodology
 - aggregation over lanes
 - cumulative flow analysis
 - discharge just downstream, if needed aggregated main+off-ramp
 - bottleneck activation / de-activation by occupancy just upstream

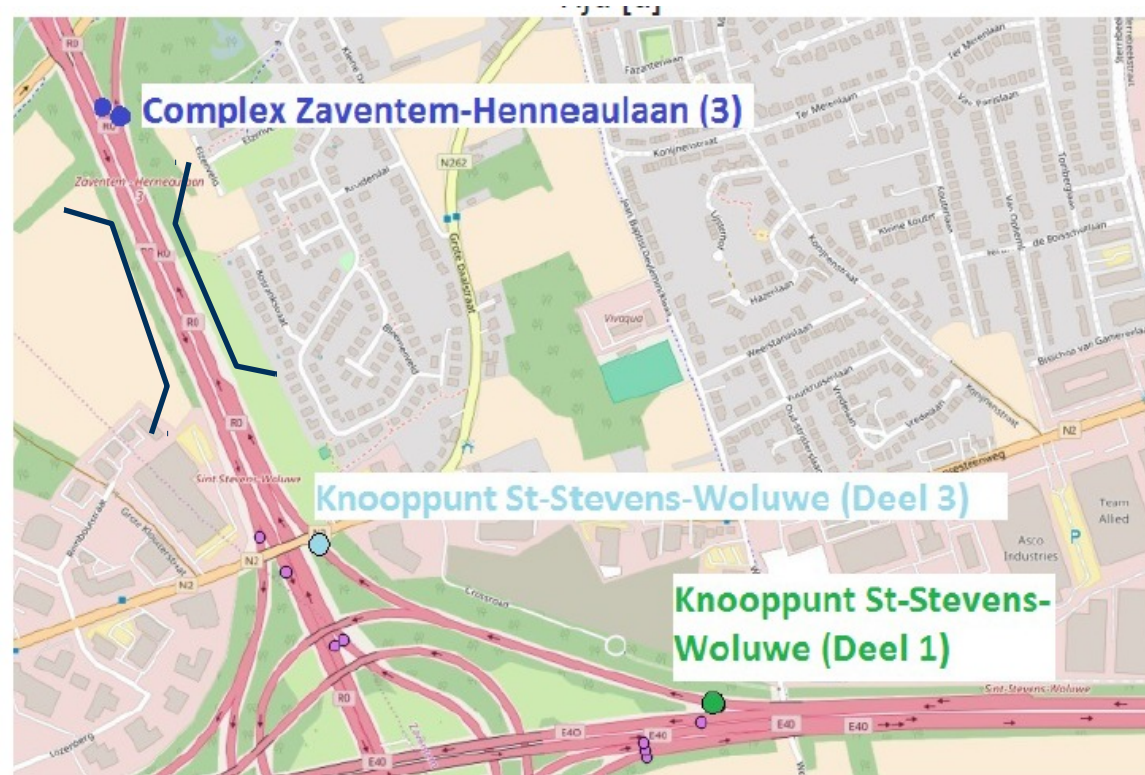
Capacity drop

- occupancy 1st upstream detector
 - threshold site specific
 - determines discharge period
- average flow during discharge period = discharge rate
- longest “constant” flow period prior \Rightarrow pre-queue capacity



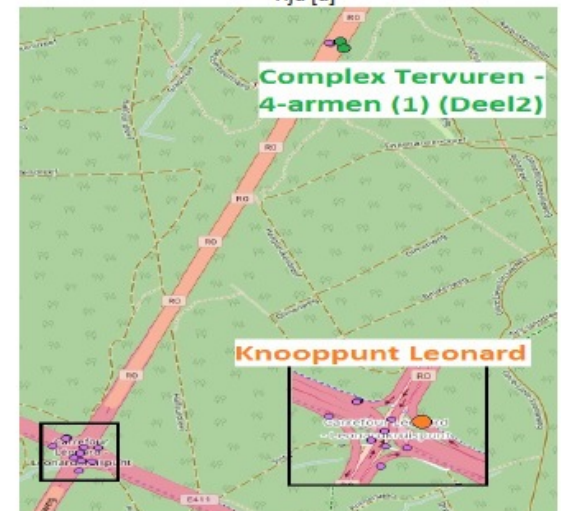
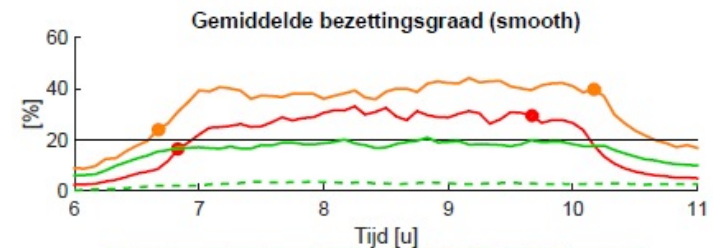
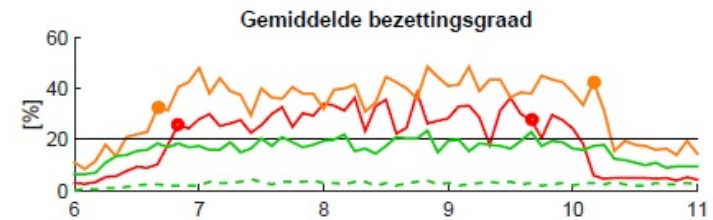
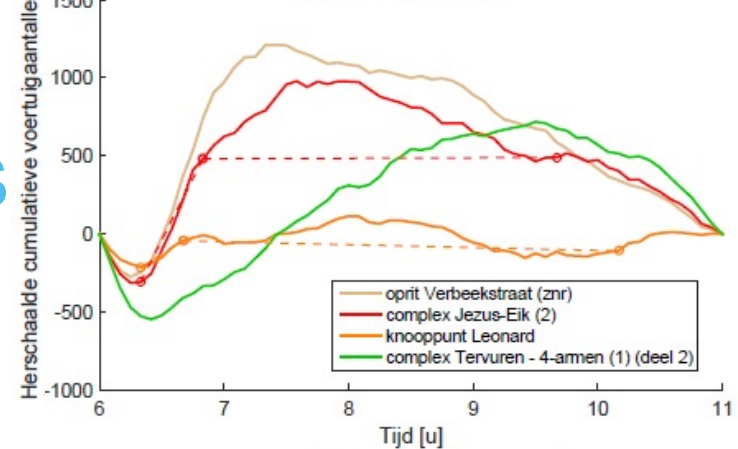
Result example: E40 (East) \Rightarrow R0 (outer - North) Sint-Stevens-Woluwe AM peak

- weaving zone bottleneck
 - 3+2 lanes
 - activated 38 times
- pre-queue cap
 - 7970 veh/h
- discharge rate
 - 7528 veh/h
- drop
 - 5.52%



Examples special cases

- bottleneck in on-ramp *before* the merge (only on merge)
 - no downstream observation
 - detector on merge (i.e. just in queue) used
- E19 (North) \Rightarrow R0 (inner – South) Machelen AM peak
 - pre-queue: 3585 veh/h
 - discharge: 3303 veh/h
 - drop: 7.8%
- E411 (SE) \Rightarrow R0 (outer – N) Leonard interchange AM
 - pre-queue: 2604 veh/h
 - discharge 1995 veh/h
 - drop: 23.3%
 - \Rightarrow to be distrusted, instable discharge (no cause found)



Overview results

	Stroom voor	Stroom na	CD [%]	Stdev(CD)	n	Starttijd
Bottleneck 1: complex Zaventem-Henneaulaan	7970	7528	-5.52	2.61	38	7u51
Bottleneck 2*: complex Machelen (E19 → binnenring)	3585	3303	-7.81	3.65	28	6u48
Bottleneck 3: knooppunt Machelen (binnenring)	6105	5919	-3.03	0.99	21	7u30
Bottleneck 4: complex UZ-Jette (binnenring)	6454	6119	-5.16	2.43	31	6u33
Bottleneck 6a: complex Hoeilaart (Leonardkruispunt)	3924	3710	-5.38	2.43	33	7u33
Bottleneck 6b*: Leonardkruispunt (E411 → buitenring)	2604	1995	-23.34	4.72	39	6u37
Bottleneck 7: complex UZ-Jette (buitenring)	6728	6449	-4.17	2.18	15	13u47
Bottleneck 8: knooppunt Machelen (buitenring)	7185	7009	-2.56	1.99	6	14u20

Conclusion and implication

- Some bottlenecks allow for CD estimation, which reproduces reliably
 - 2.5 – 5.5% (range over sites)
 - 0 – 16% (range over days and sites)
- Others give unreliable estimates
- Others exhibit no noticeable drop
- Implication:
“Does not seem like a sound basis to recommend CD-avoiding traffic control operations like ramp metering or MTFC?”
- Would you agree or am I missing something?

Capacity drop on Belgian intersections

Master thesis Martin Camerman
KU Leuven master Traffic & Logistics

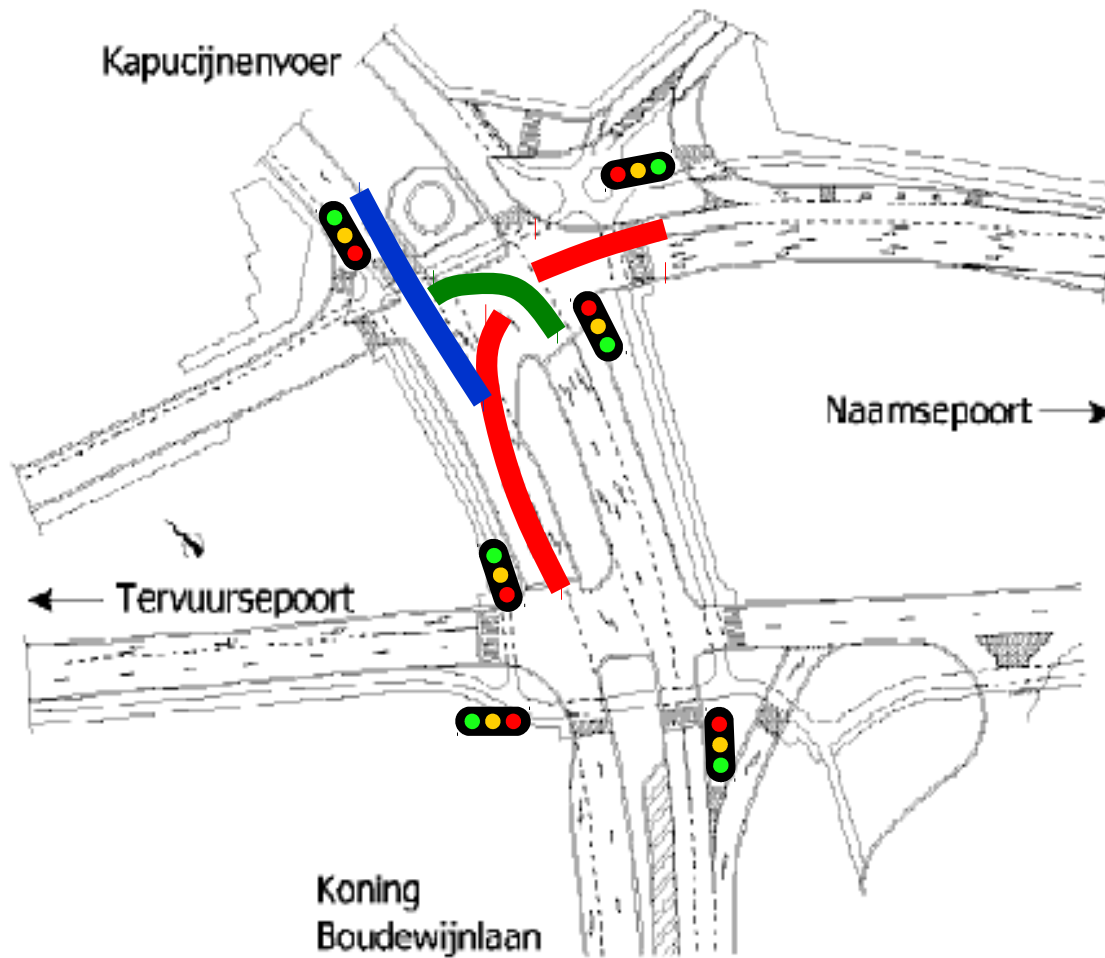
Capdrop of intersections - definition

“Capacity drop is the reduction in throughput for a movement over the intersection, caused by conflicts that were underestimated during design of the traffic control, or that were supposed to be excluded by traffic control but still activate due to problems in evacuating the intersection.”

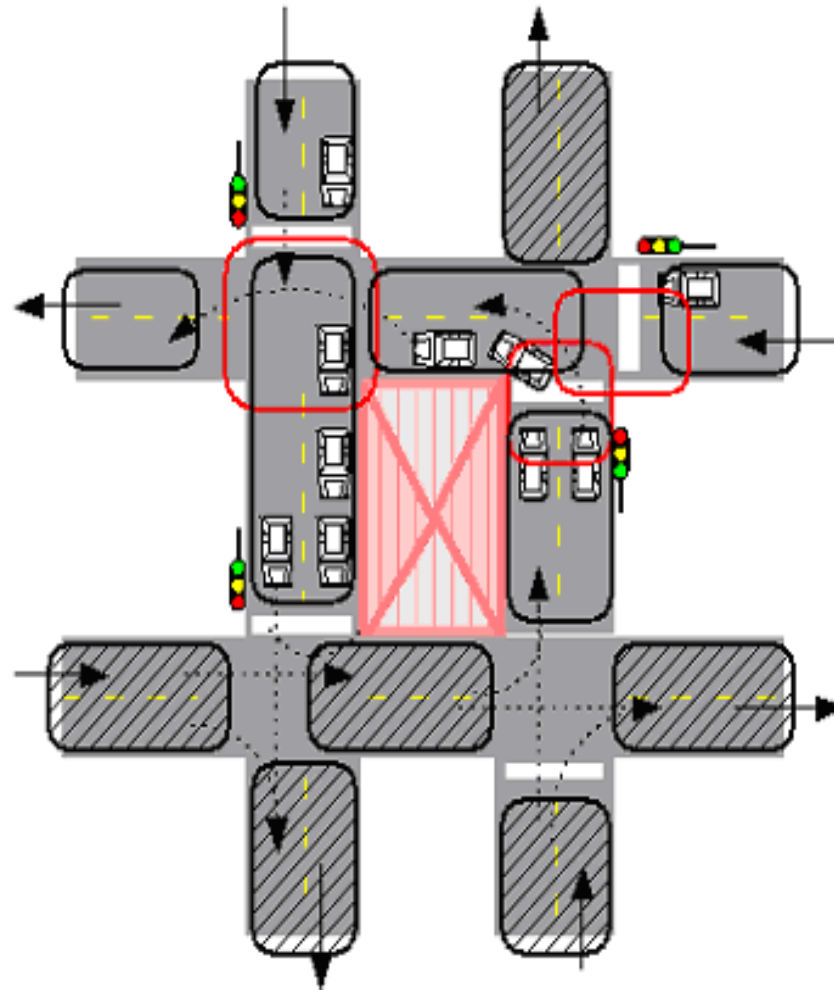
Let's go back 10 years...

- my presentation at masterclass November 2006 – PhD Francesco Viti

Application

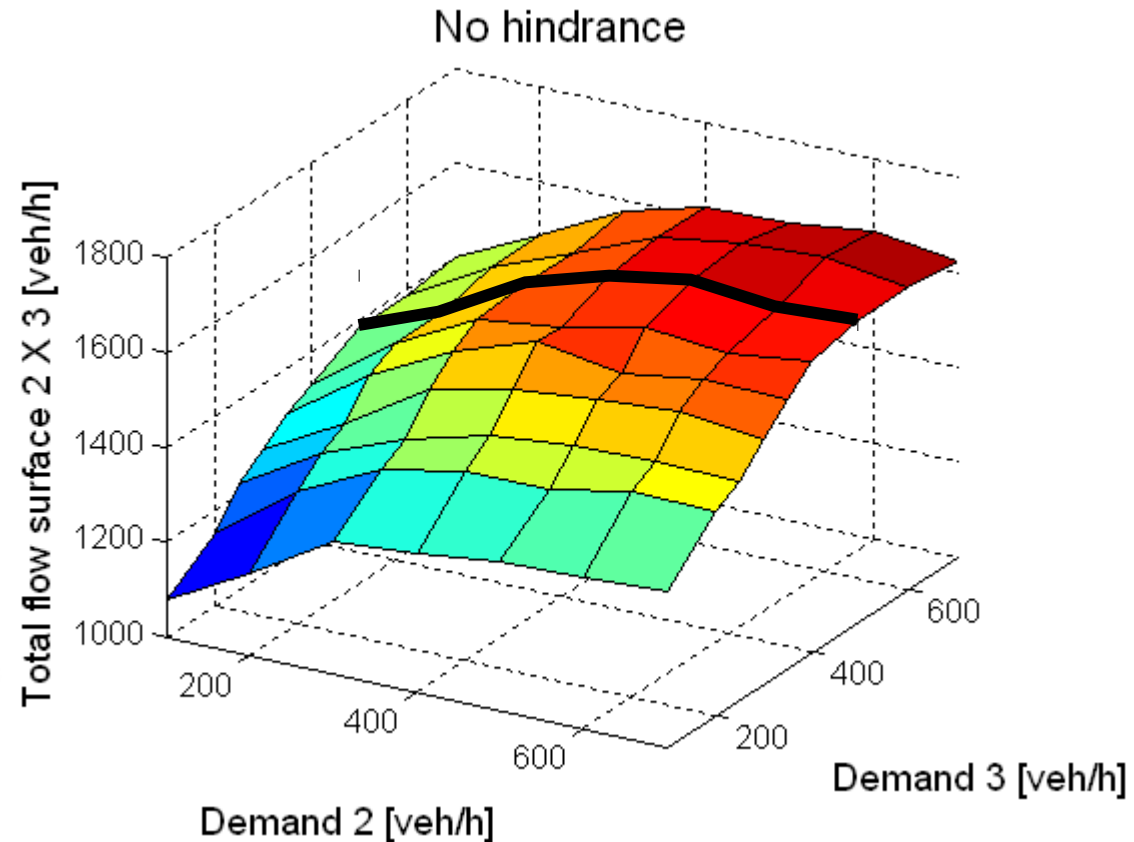
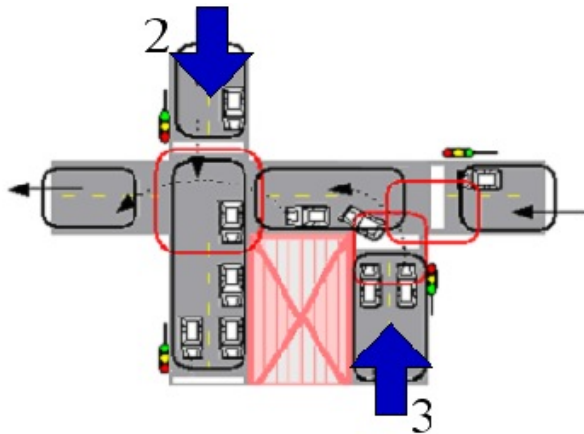


Modelling the network

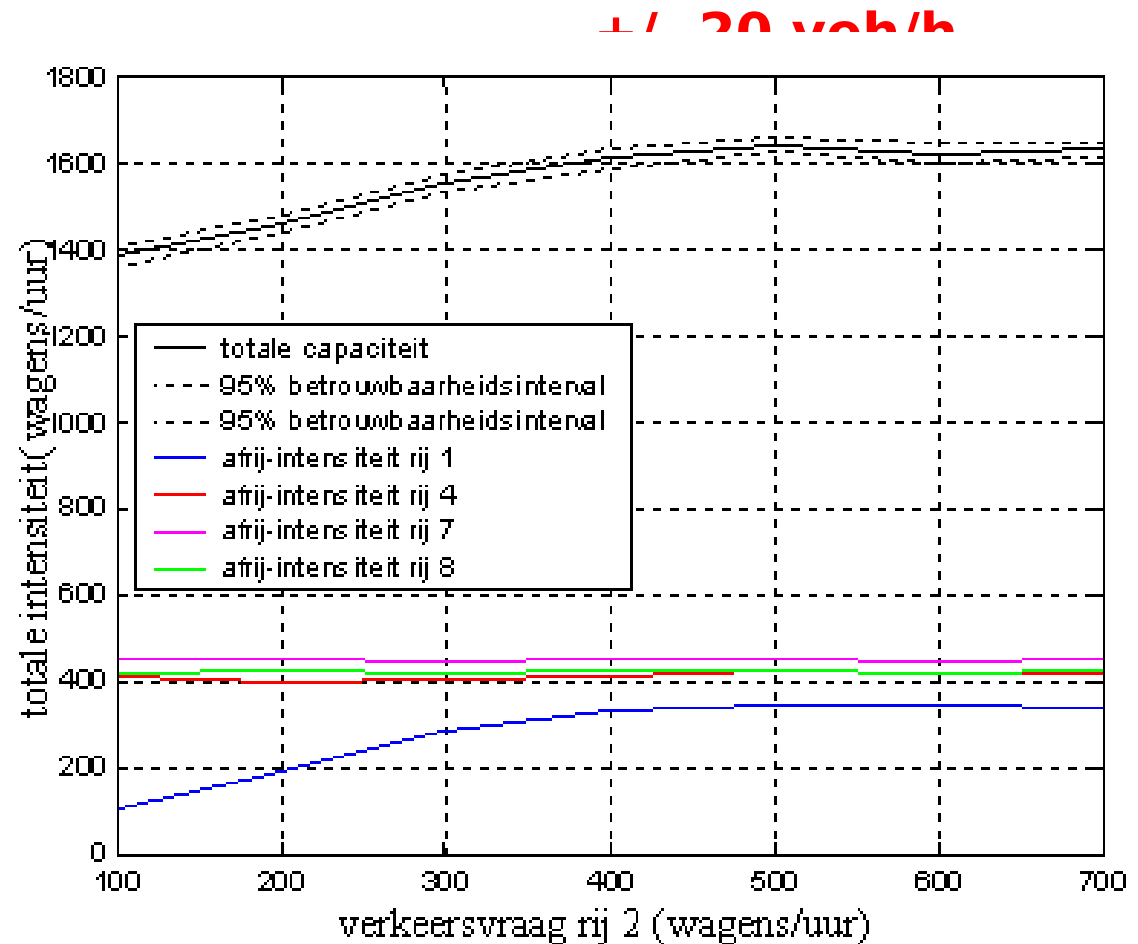
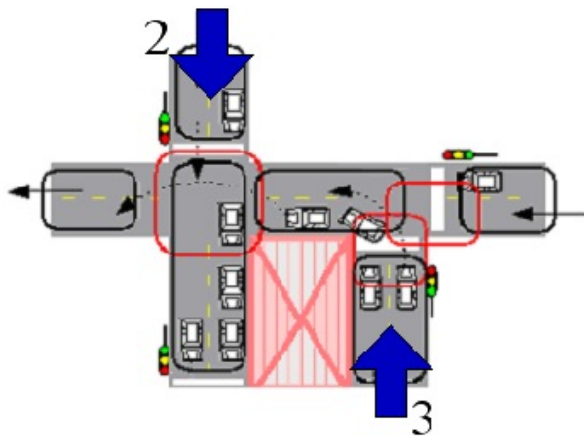


Simulation results: reference without hindrance

- averaged over 50 15' runs for each gridpoint
- other demands 500 veh/h

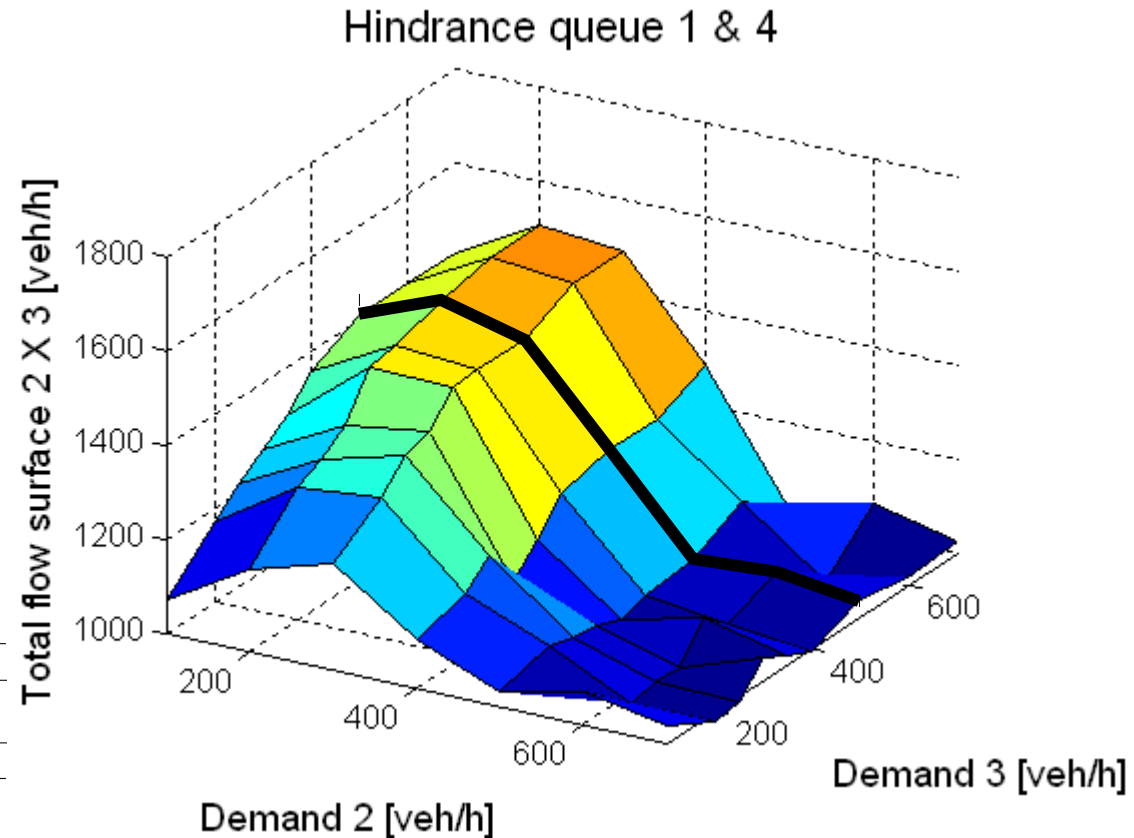
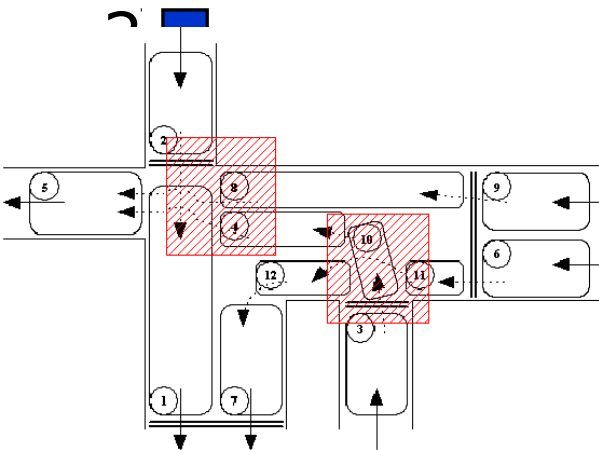


95% confidence interval and composition

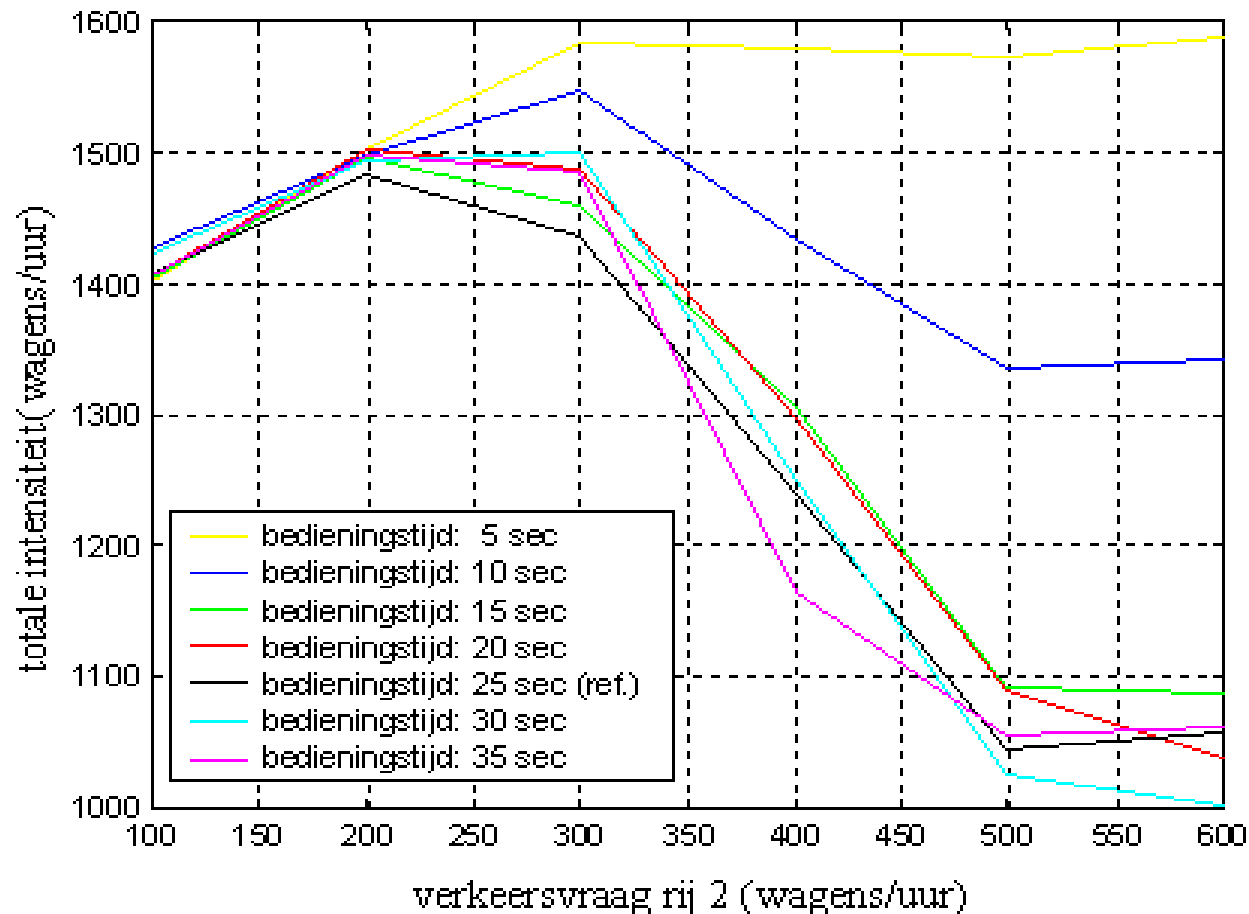


Simulation results: capacity drop!

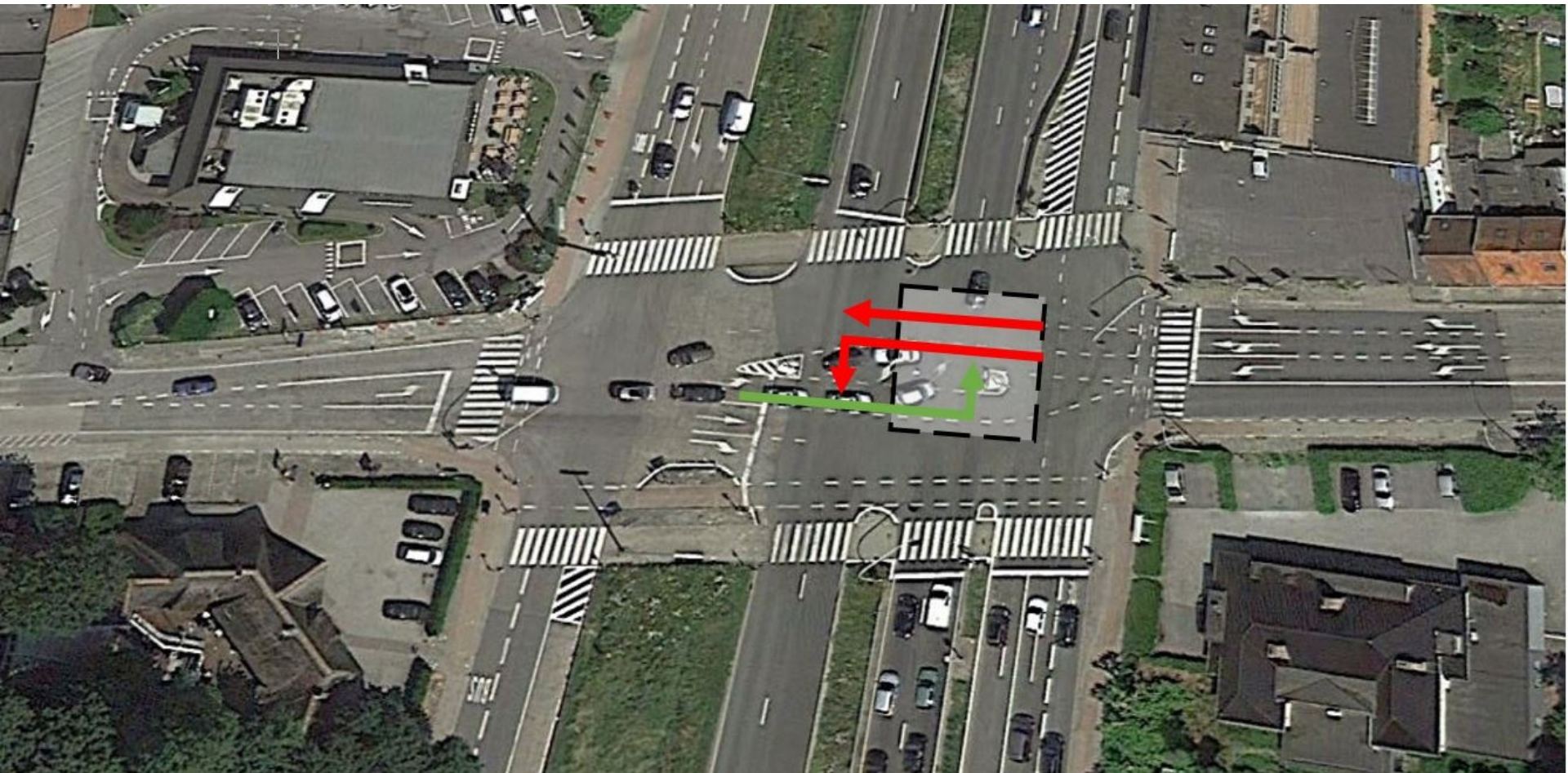
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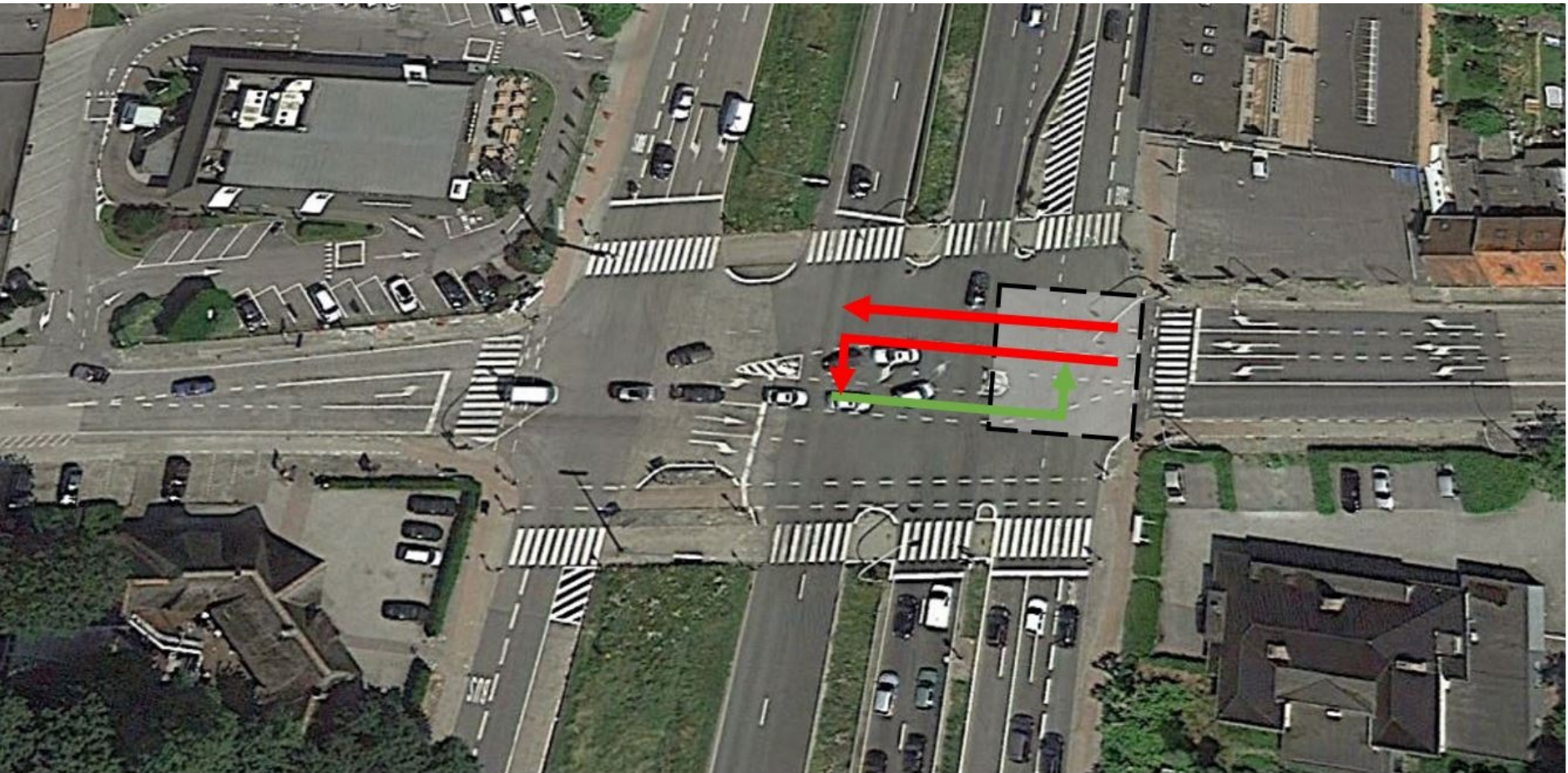
sensitivity analysis: influence service time increase when obstructed



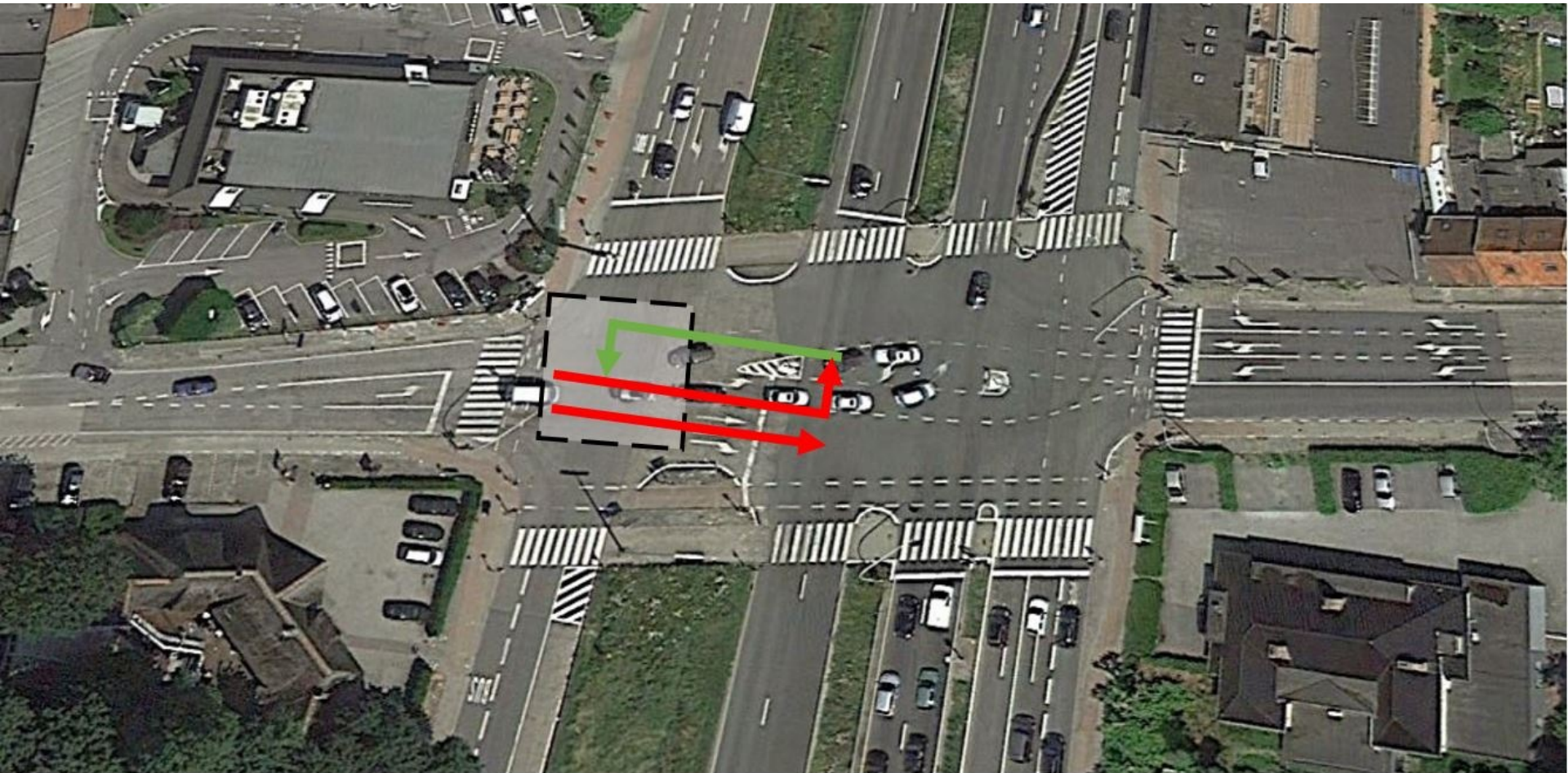
Boomsesteenweg – L1



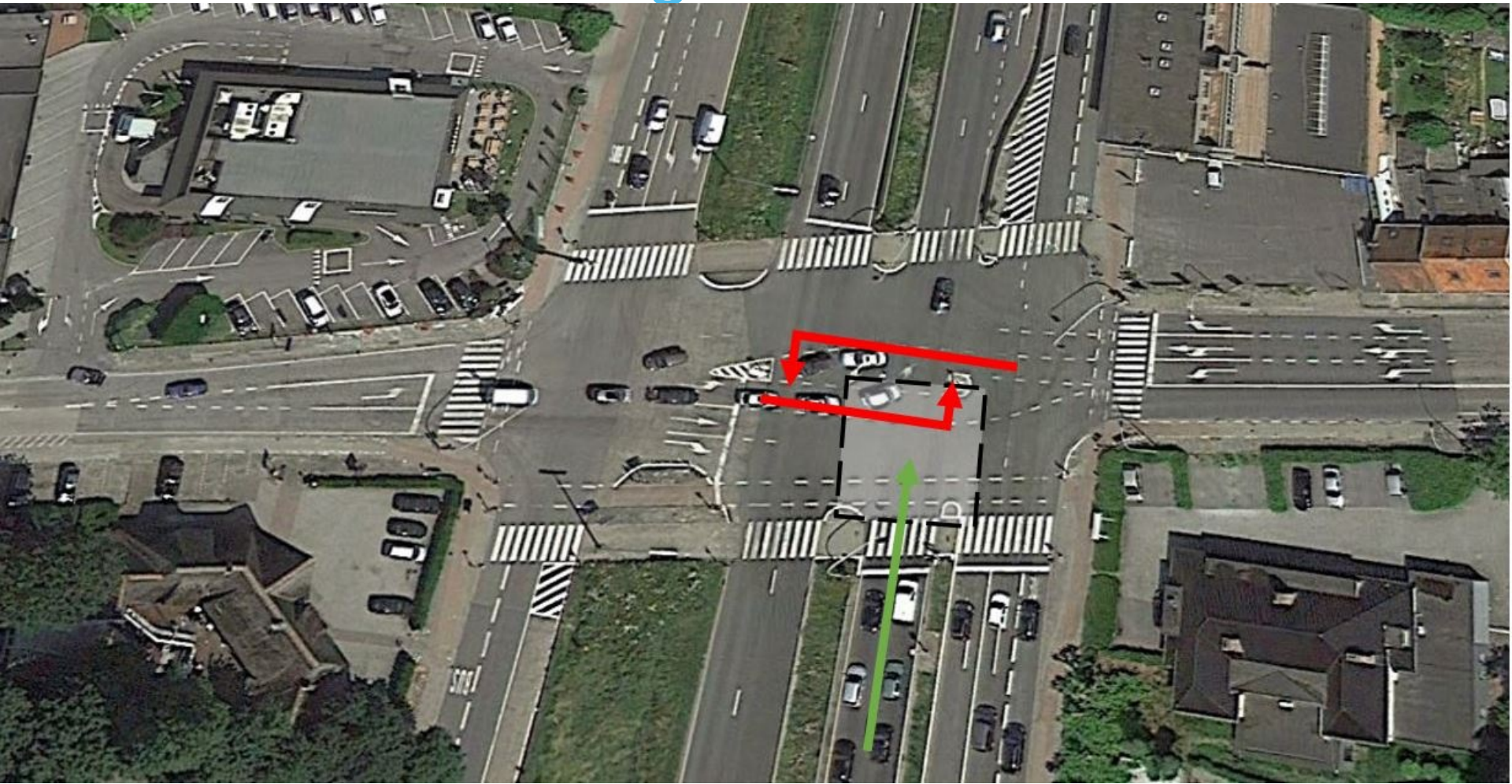
Boomsesteenweg – L2



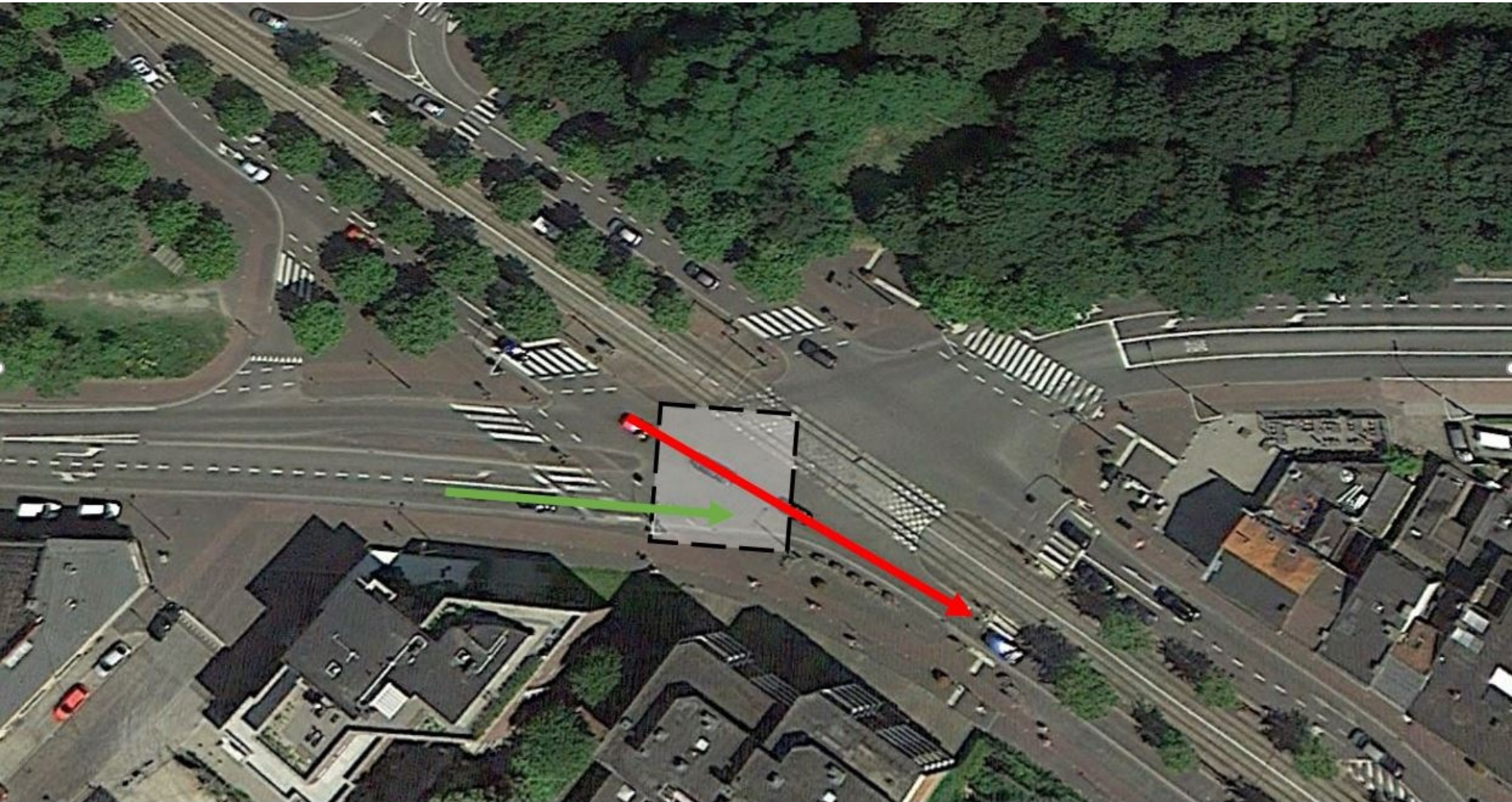
Boomsesteenweg – L3



Boomsesteenweg – R1



Mortsel – R2



Dynamische effecten - block



Dynamische effecten - push



Dynamische effecten - squeeze



Conflict 'linksaf' - referentiecapaciteit

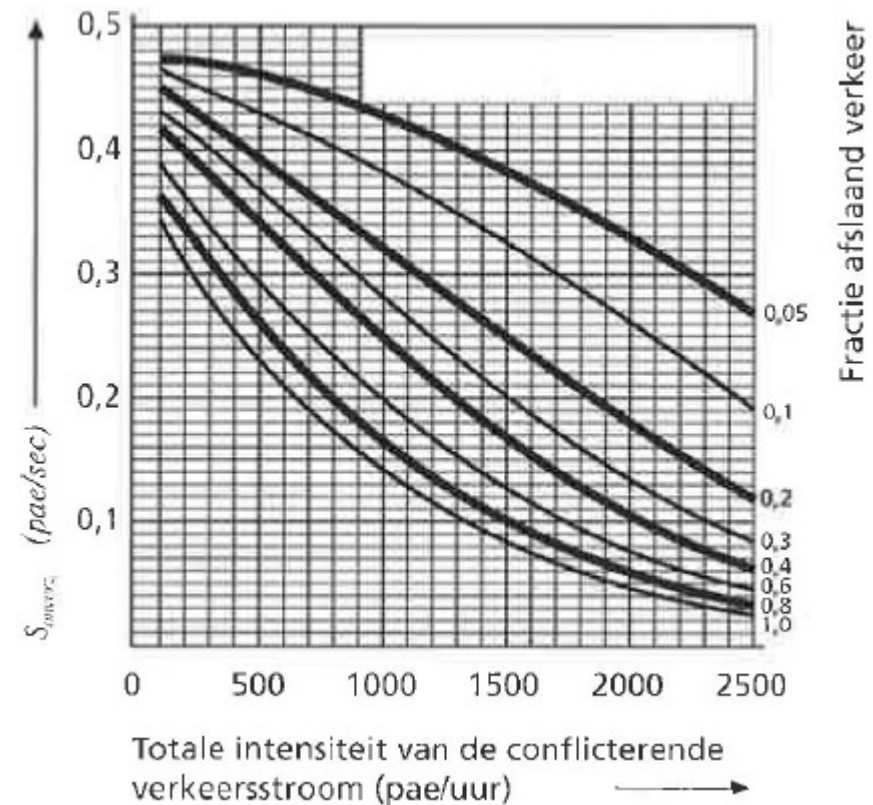
~ CAPCAL methode

$$PAE_{i,unsat} = \frac{G_{eff}}{T_{cyclus}} \cdot PAE_{i,tot}$$

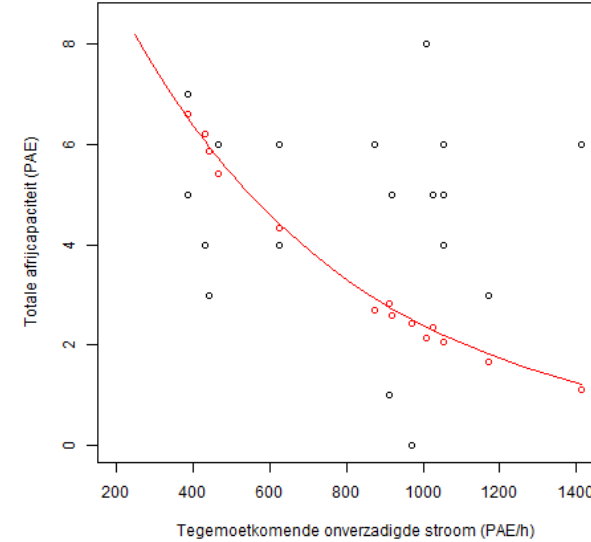
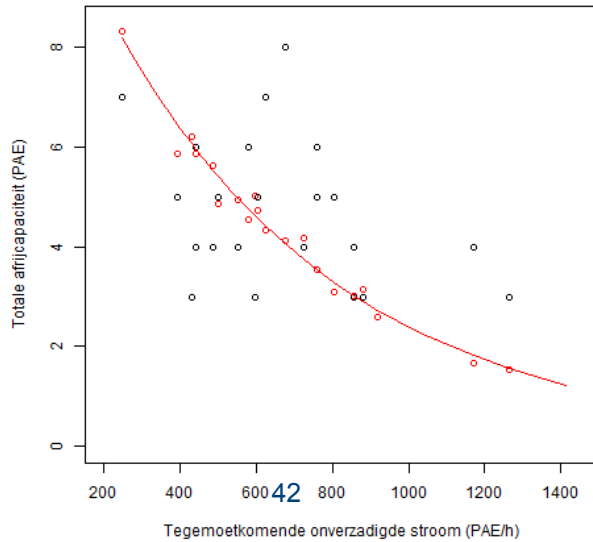
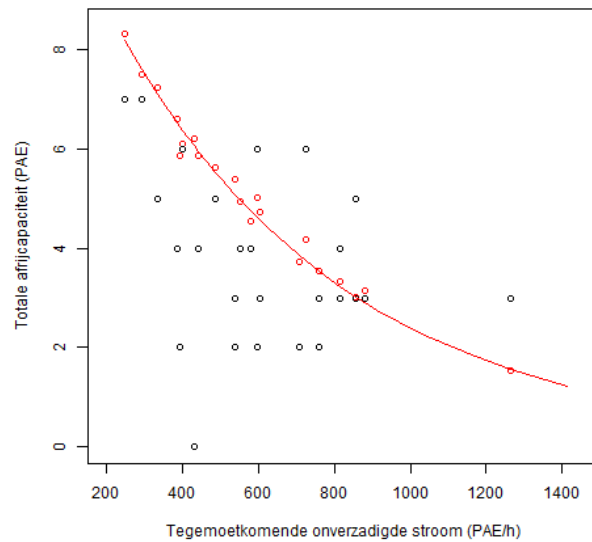
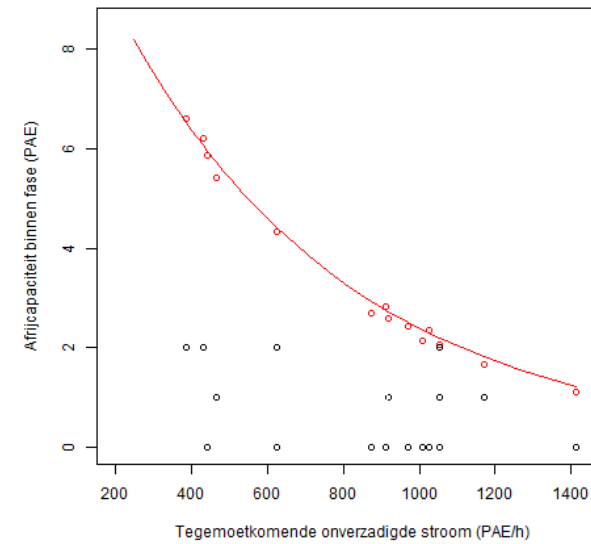
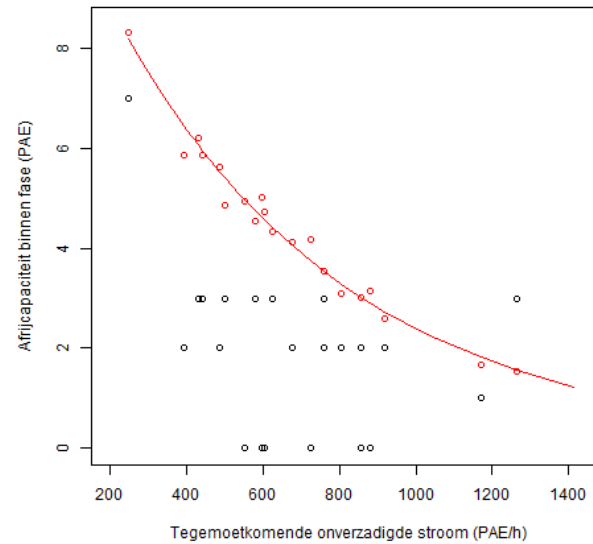
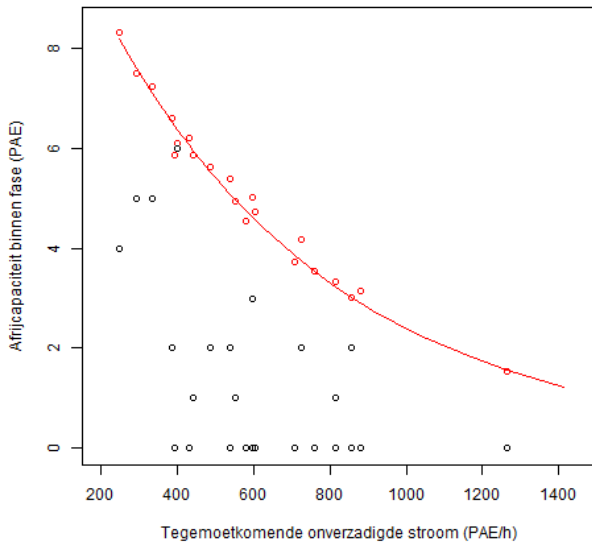
$$G_{i,sat} = \frac{3600(s/h)}{1800(PAE/h)} \cdot PAE_{i,sat}$$

$$Flow_{i,unsat} = \frac{PAE_{i,unsat}}{G_{i,unsat}}$$

$$Cap_{ref} = Cap_s \cdot \min(G_{i,unsat})$$



Conflict 'linksaf' – gemeten vs. referentie



Conflict 'linksaf' – regressie totaal

	Constante	c_{block}	$c_{Squeeze}$	c_{Push}	c_{Spill}	$c_{PAE_{left}}$	$c_{Flow_{unsat}}$	c_{Opstel}	R^2	R_A^2
L1	8.876	-0.066			nvt.	-2.589	180.532	nvt.	78.32	75.72
L2	7.420	-0.053		3.691	nvt.		-8.271	nvt.	65.52	60.35
L3	5.611	-0.055	1.389		-2.929			nvt.	77.51	73.02
Totaal	3.820	-0.069	0.343	0.755	-2.867			0.091	69.89	67.60

- $$T_{block} = A + c_{PAE} \cdot PAE + c_{PAE_{left}} \cdot PAE_{left,tot}$$

	Constante	PAE	$PAE_{left,tot}$	R^2	R_A^2
L1	47.623	-8.812	2.593	77.82%	76.11%
L2	0.627	-3.613	6.010	70.51%	67.70%
L3	-26.076	-1.383	8.512	63.42%	58.84%
Totaal	19.086	-5.876	4.924	68.09%	67.17%

- Dynamic effects (Squeeze, push) influence significantly but not predicted themselves

Conclusions of this first exploration

- Literature: unrealistic performance of saturated intersections
 - need for theory and observations of capacity drop of intersections
- Proposed definition of capacity drop intersections
- Exploration of role of 3 dynamic effects
 - Block
 - Push
 - Squeeze
- Some empirical results prove their influence on intersection discharge, along with
 - (time blocked)
 - volumes of conflicting flows

Can I consult you?

- Do you think my conclusion about capacity drop on R0 Brussels is justified?

“Empirical results do not appear to be a sound basis to recommend CD-avoiding traffic control operations like ramp metering or MTFC?”

- Do you think capacity drop at intersections is a topic worth further consideration?
 - if so, which aspects? how?

Questions?

